

## TEACHING STRUCTURAL STEEL DESIGN USING MATHCAD PROGRAM

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### ABSTRACT

This paper demonstrates the integration of Mathcad programming in a steel design course at the University of Qatar. It discusses the advantages of Mathcad programming over other programming formats and provides guidelines to incorporate Mathcad programming into the steel design course. A Mathcad program for the analysis and design of steel beams is presented to show the attractive computational environment of Mathcad. The importance of Mathcad programming in teaching steel design courses is also illustrated. Successful integration of Mathcad programming as a teaching and learning tool in the steel design course resulted in an increased students' understanding of structural analysis and design.

**KEY WORDS:** Mathcad, Learning Tool, Teaching Tool, Structural Steel Design, High-Level Programming.

### INTRODUCTION

Computers have become indispensable analysis and design tools for engineers because they are capable of producing massive amounts of visual data. A large number of commercial computer programs are currently available to solve engineering problems. This situation led computer users to forget that they have to be always familiar with the theory and assumptions behind these programs. This problem is especially significant for undergraduate engineering students who probably do not completely understand the theory.

One solution to this problem consists of having students write their own analysis

and design programs using a high-level programming language such as FORTRAN or C++. This approach is not desirable for two reasons. The effort required on the part of the students would exceed the benefit they might achieve relative to the course objectives. Also, the level of programming expertise of students at the undergraduate level is not adequate.

A better solution consists of having students write their own analysis and design programs using a commercial software platform such as Mathcad [1], which is an efficient learning environment for technical topics such as engineering design. The computational and presentation capabilities of Mathcad allow for the solution of mathematically based problems and for the effective communication of both problem and solution. It contains powerful presentation capabilities, which include the use of charts, graphic objects, and animation effects. It can also easily import objects from other application programs, such as images and digital photographs. These capabilities offer significant learning enhancements to engineering students. Mathcad makes possible new learning strategies for students and instructors. What-if discussions, trend analyses, trial and error analyses, and optimization are all valuable learning activities. Taking advantage of the computational power and speed of Mathcad, instructors and students can quickly cycle through design problem scenarios. It allows students to apply the solution logic without the programming difficulties and overhead associated with high-level language programming. Mathcad has proven to be an excellent teaching and learning tool for reinforced concrete design[2].

The proposed paper describes the integration of Mathcad programming in a steel design course. It discusses advantages of Mathcad programming and provides recommendations for incorporating Mathcad programming in the steel design course. A Mathcad program for the analysis and design of steel beams is presented to show the attractive computational environment of Mathcad. The importance of Mathcad programming in teaching steel design courses is also illustrated.

## COURSE DESCRIPTION

Structural Steel Design is an introductory steel design course, which is required for civil engineering students specializing in structures. There are three lecture hours per week and one weekly two-hour tutorial session. The course uses the American Institute of Steel Construction Load and Resistance Factor Design (LRFD) methodology throughout [3]. Six distinct blocks or topics are covered within the course including structural systems, tension members, compression members, flexural

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members, beam-columns, and connections. During the semester, the students must complete an engineering design project involving the design of a simple structural system.

### **ADVANTAGES OF MATHCAD PROGRAMMING**

Because it eliminates much of the programming overhead, Mathcad is superior to high-level languages. Mathcad is relatively easy to learn and straightforward and at the same time offers powerful tools to create sophisticated programs. More accomplished students can make advanced Mathcad programs while less experienced students can still write a simple Mathcad program that gets the job done.

Mathcad programming logic closely resembles the logical flow of the engineering thought process. In Mathcad, the equations used to represent the engineering thought process look the same as they are written in a reference book. Once the equations are entered into the program, it is easy to check the validity of the logic because the calculations are immediate. As a result, there is an obvious relationship between the engineering thought process, the equations needed to represent that thought process, and the iteration through those equations to achieve an optimal solution.

In Mathcad program, different formatting, including various fonts, colors, patterns, and borders can be used to improve the readability of the text. By using different drawing entities and varying their color, pattern, and line weight attributes, highly readable drawings are produced to illustrate the computations.

Mathcad program provides outstanding graphics capabilities. The way in which Mathcad tend to relate numbers to graphics is important. Since Mathcad can generate graphs from a range of numerical values, it is easy to generate a graphical depiction of a solution. Furthermore, it is possible to directly alter the graphical output by changing the desired parameters. Like spreadsheets, as soon as a change is made in the input data, the results are updated and the plots are redrawn. Other types of charts, such as pie and histogram charts, can also be easily generated. The Mathcad program allows for the determination of an optimum design simply by changing the input data and observing the changes in the design.

### **GENERAL GUIDELINES FOR INCORPORATING MATHCAD**

The computer usage in undergraduate courses aims at helping students to learn the

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actual course objectives. Therefore, the effort required to learn the software package should not eclipse the student's effort devoted to learning the course objectives.

When using Mathcad students should start by outlining their solution thought process using a complete hand solution. This is important for two reasons. First, it ensures that students work through the theory. Second, it improves their chances of entering the correct equations into the program. Students need to generate some type of graphic output from their Mathcad program. This is important because it causes them to visualize the effect of their decisions. Students should be reminded that the computer program is merely a tool. They must always evaluate the computer output. They should use Mathcad program for the exploration of alternate problem scenarios, observation of trends, and expansion of the discussion to related topics. The time spent using the program to explore problem scenarios can lead students to a better understanding of the concepts involved in the problems.

### MATHCAD PROGRAM FOR STRUCTURAL STEEL ANALYSIS AND DESIGN

The key to incorporate Mathcad in the steel design course is to constantly reinforce the notion that it is the engineer not the computer who must eventually solve the problem. The computer is merely a tool.

A Mathcad program, which has been incorporated into a steel design course, is discussed. The program, which is concerned with the design of structural steel beams, was written to automate the manual design procedure [4-7]. The Mathcad program consists of the following computational steps:

#### STEP 1.

The first step consists of reading the required beam input data (Figure 1).

#### STEP 2.

The second step involves the determination of the ultimate bending moment  $M_u$  (Figure 2). The number and positions of the concentrated (point) loads are not fixed in order to create a generic program.

#### STEP 3.

The third step involves the determination of the beam moment gradient  $C_b$  (Figure 3).

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### STEP 4.

The fourth step consists of computing the design bending moment  $M_n$  (Figure 4).

### STEP 5.

The fifth step consists of displaying the analysis results and drawing the beam bending moment diagram (Figure 5).

## ILLUSTRATIVE EXAMPLE

This example is presented to demonstrate the analysis and design features of the Mathcad program. The example consists of the design of a simply-supported structural steel beam with a span of 30 ft. The beam is subjected to a uniform dead load of 0.31 kip/ft and to a uniform live load of 1.0 kip/ft. The objective of the design is to select the lightest (i.e., most economical) W-shape that resists these loads. The first W-shape considered (W10\*77) was selected using the  $Z_x$  table of the LRFD Book. The input data and output results for the first design trial is summarized in Figure 6. The results show that the first design is not economical (i.e.,  $M_n \gg M_u$ ).

The program is easily used to improve the first design trial by selecting a smaller and lighter W-shape. In the second trial, the shape W10\*49 is selected. The input data and output results for the section design trial are summarized in Figure 7. Since the design strength of the section is less than the ultimate moment (i.e.,  $M_n < M_u$ ), the selected shape is not adequate. In the third design trial, the W12\*58 is selected. The input data and output results for the third design trial is summarized in Figure 8. The results show that an optimum design was easily reached after only two trials. This shows the efficiency of the Mathcad program presented.

STEEL BEAM DESIGN USING LRFD METHOD

STEP 1. READ INPUT DATA

**Read material properties data**

Fy = 50 ksi      Fr = 10      ksi      Yield and residual strength  
 E = 29000 ksi      G = 11200      ksi      Elastic and shear modulus

**Read section properties data**

h = 14.0      in      A = 26.5      in<sup>2</sup>      ow = 0.09       $\frac{\text{kips}}{\text{ft}}$       Beam height, area, and own weight  
 Bf = 14.520      in      Tf = 0.710      in      Flange width and thickness  
 Ix = 999      in<sup>4</sup>      Sx = 143      in<sup>3</sup>      Zx = 157      in<sup>3</sup>      Moment of inertia, section modulus, and plastic modulus (about x-axis)  
 Iy = 362      in<sup>4</sup>      J = 4.06      in<sup>4</sup>      Cw = 16000      in<sup>3</sup>      Moment of inertia (about y-axis), torsional constant, and warping constant

**Read geometry data**

L = 40      ft      Beam span  
 Lb = 40      ft      Braced length  
 Lb1 = 0      ft      Lb2 = 40      ft      Beam lateral bracing positions (left and right)

**Read load data**

M1 = 0      ft kips      M2 = 0      ft kips      Left and right- end moments  
 DL = 0.31       $\frac{\text{kips}}{\text{ft}}$       LL = 1.0       $\frac{\text{kips}}{\text{ft}}$       Beam uniform dead and live loads  
 NCL = 1      Number of concentrated loads  
 AA := READPRN ("test.dat" )  
 P := AA <1>      l := AA <0>

Fig.1. Step 1 of Mathcad program

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### STEP 2. DETERMINATION OF ULTIMATE BENDING MOMENT $M_u$

Compute the beam reaction  $R_1$  due to the uniform load ( $w$ ), the concentrated loads ( $P_k$ ), and the end-moments ( $M_1$  &  $M_2$ )

$$w := 1.2 \cdot (DL + ow) + 1.6 \cdot LL$$

$$k := 0..NCL$$

$$R_1 := \frac{w \cdot L}{2} + \left[ \sum_{k=0}^{NCL-1} \left[ \frac{P_k \cdot (L - l_k)}{L} \right] \right]$$

$$R_1 := R_1 - \frac{M_1 + M_2}{L}$$

Compute the beam ultimate bending moment  $M_u$

$$i := 0..L \quad j := 0..0$$

$$x_1 := i$$

$$xx_{1,j} := \begin{cases} x_1 - l_j & \text{if } x_1 - l_j \geq 0 \\ 0 & \text{otherwise} \end{cases}$$

$$M_i := M_1 + R_1 \cdot x_1 - \sum_{j=0}^{NCL-1} P_j \cdot xx_{1,j} - \frac{w \cdot (x_1)^2}{2}$$

$$M_u := \max(M)$$

Fig.2. Step 2 of Mathcad program

**STEP 3. DETERMINATION OF MOMENT GRADIENT Cb**

Compute the maximum bending moment (Mmax) in the braced length (Lb3)

$$Lb3 := Lb2 - Lb1$$

$$k := 0 .. Lb3 \qquad j := 0 .. 0$$

$$y_k := k + Lb1$$

$$xx_{k,j} := \begin{cases} y_k - l_j & \text{if } y_k - l_j \geq 0 \\ 0 & \text{otherwise} \end{cases}$$

$$MX_k := M1 + R1 \cdot y_k - \sum_{j=0}^{NCL-1} P_j \cdot xx_{k,j} - \frac{w \cdot (x_k)^2}{2}$$

$$Mmax := \max (MX)$$

Compute the bending moment MA, MB, and MC

$$k := 0 .. 2 \qquad j := 0 .. 0$$

$$y_k := Lb1 + (k + 1) \cdot \frac{(Lb2 - Lb1)}{4}$$

$$xx_{k,j} := \begin{cases} y_k - l_j & \text{if } y_k - l_j \geq 0 \\ 0 & \text{otherwise} \end{cases}$$

$$MY_k := M1 + R1 \cdot y_k - \sum_{j=0}^{NCL-1} P_j \cdot xx_{k,j} - \frac{w \cdot (y_k)^2}{2}$$

$$MA := MY_0 \qquad MB := MY_1 \qquad MC := MY_2$$

Compute the beam moment gradient Cb

$$Cb := \frac{12.5 \cdot Mmax}{2.5 \cdot Mmax + 3 \cdot MA + 4 \cdot MB + 3 \cdot MC}$$

Fig. 3. Step 3 of Mathcad program



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### STEP 4. COMPUTE BEAM DESIGN STRENGTH ( $\Phi M_n$ )

Check flange local buckling (i.e., check if the section is compact)

$$\lambda := \frac{b_f}{2 \cdot t_f} \quad \lambda_p := \frac{65}{\sqrt{F_y}} \quad \lambda_r := \frac{141}{\sqrt{F_y - 10}}$$

$$\text{FLB} := \begin{cases} 0 & \text{if } \lambda < \lambda_p \\ 1 & \text{if } \lambda_p < \lambda \leq \lambda_r \\ 2 & \text{if } \lambda > \lambda_r \end{cases} \quad \begin{array}{l} \text{Shape is compact} \\ \text{Shape is noncompact} \\ \text{Shape is slender} \end{array}$$

Check lateral torsional buckling

$$R_y := \sqrt{\frac{I_y}{A}}$$

$$X1 := \frac{\pi \cdot \sqrt{\frac{E \cdot G \cdot J \cdot A}{2}}}{S_x}$$

$$X2 := 4 \cdot \frac{C_w \cdot \left(\frac{S_x}{G \cdot J}\right)^2}{I_y}$$

$$L_p := 300 \cdot \frac{R_y}{12 \cdot \sqrt{F_y}} \quad L_r := \frac{R_y \cdot X1 \cdot \sqrt{1 + \sqrt{1 + X2 \cdot (F_y - F_r)^2}}}{12 \cdot (F_y - F_r)}$$

$$\text{LTB} := \begin{cases} 0 & \text{if } L_b \leq L_p \\ 1 & \text{if } L_p < L_b \leq L_r \\ 2 & \text{if } L_b > L_r \end{cases} \quad \begin{array}{l} \text{No risk of lateral torsional buckling} \\ \text{Risk of inelastic lateral torsional buckling} \\ \text{Risk of elastic lateral torsional buckling} \end{array}$$

Compute plastic moment ( $M_p$ )

$$M_p := \begin{cases} \frac{F_y \cdot Z_x}{12} & \text{if } \left(\frac{Z_x}{S_x}\right) \leq 1.5 \\ \frac{(1.5 \cdot F_y \cdot S_x)}{12} & \text{if } \left(\frac{Z_x}{S_x}\right) > 1.5 \end{cases} \quad \text{Plastic moment}$$

$$\phi M_p := 0.9 \cdot M_p$$

$$M_r := \frac{(F_y - F_r) \cdot S_x}{12} \quad \text{Moment corresponding to first yield}$$

**Fig. 4. Step 4 of Mathcad program**

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$$M_{n1} := C_b \left[ M_p - (M_p - M_r) \frac{(L_b - L_p)}{(L_r - L_p)} \right]$$

$$M_{n2} := \begin{cases} M_{n1} & \text{if } M_{n1} \leq M_p \\ M_p & \text{otherwise} \end{cases}$$

Nominal strength  $M_n$  (compact + inelastic torsional buckling)

$$M_{cr} := \frac{C_b \cdot S_x \cdot X_1 \cdot \sqrt{2} \cdot \sqrt{1 + \frac{X_1^2 \cdot X_2}{2 \cdot \left(\frac{12 \cdot L_b}{R_y}\right)^2}}}{12 \cdot \frac{12 \cdot L_b}{R_y}}$$

$$M_{n3} := \begin{cases} M_{cr} & \text{if } M_{cr} \leq M_p \\ M_p & \text{otherwise} \end{cases}$$

Nominal strength  $M_n$  (compact + elastic torsional buckling)

$$M_{n4} := \begin{cases} M_p & \text{if } LTB=0 \\ M_{n2} & \text{if } LTB=1 \\ M_{n3} & \text{if } LTB=2 \end{cases}$$

Nominal strength  $M_n$  for compact sections

$$M_{n5} := \left[ M_p - (M_p - M_r) \frac{(\lambda - \lambda_p)}{(\lambda_r - \lambda_p)} \right]$$

$$M_{n6} := \begin{cases} M_{n4} & \text{if } M_{n5} \geq M_{n4} \\ M_{n5} & \text{otherwise} \end{cases}$$

Nominal strength  $M_n$  for noncompact sections

$$\phi M_n := \begin{cases} (0.9 \cdot M_{n4}) & \text{if } FLB=0 \\ (0.9 \cdot M_{n6}) & \text{if } FLB=1 \end{cases}$$

Design strength  $\phi M_n$

Fig. 4. Step 4 of Mathcad program (continued)

## Teaching Structural Steel Design Using Mathcad Program

### STEP 5. OUTPUT RESULTS

|                             |                    |                     |                         |
|-----------------------------|--------------------|---------------------|-------------------------|
| $L_b = 40$ ft               | $L_p = 13.07$ ft   | $L_r = 38.4$ ft     |                         |
| $\lambda = 10.23$           | $\lambda_p = 9.19$ | $\lambda_r = 22.29$ |                         |
| $\phi M_p = 588.75$ ft kips |                    |                     | Plastic bending moment  |
| $M_u = 416$ ft kips         |                    |                     | Ultimate bending moment |
| $\phi M_n = 462.02$ ft kips |                    |                     | Design bending moment   |

"The beam is adequate  $\phi M_n \geq M_u$ "

"The beam is not adequate  $\phi M_n < M_u$ "

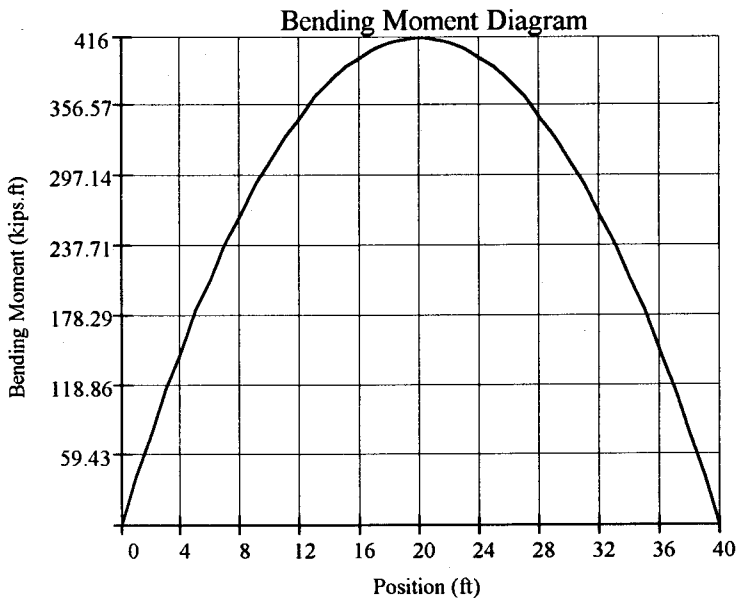


Fig. 5. Step 5 of Mathcad program

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## INPUT DATA

### Material properties data

|                   |     |                   |     |                             |
|-------------------|-----|-------------------|-----|-----------------------------|
| $F_y \approx 50$  | ksi | $F_r \approx 10$  | ksi | Yield and residual strength |
| $E \approx 29000$ | ksi | $G \approx 11200$ | ksi | Elastic and shear modulus   |

### Section properties data

|                     |                 |                     |                 |                    |                                 |  |
|---------------------|-----------------|---------------------|-----------------|--------------------|---------------------------------|--|
| $d \approx 10.6$    | in              | $A \approx 22.6$    | in <sup>2</sup> | $ow \approx 0.077$ | $\frac{\text{kips}}{\text{ft}}$ | Beam height, area, and own weight  |
| $B_f \approx 10.19$ | in              | $T_f \approx 0.870$ | in              |                    |                                 | Flange width and thickness   |
| $I_x \approx 455$   | in <sup>4</sup> | $S_x \approx 85.9$  | in <sup>3</sup> | $Z_x \approx 97.6$ | in <sup>3</sup>                 | Moment of inertia, section modulus, and plastic modulus (about x-axis)     |
| $I_y \approx 154$   | in <sup>4</sup> | $J \approx 5.11$    | in <sup>4</sup> | $C_w \approx 3630$ | in <sup>6</sup>                 | Moment of inertia (about y-axis), torsional constant, and warping constant |

## OUTPUT RESULTS

|                     |         |                    |    |                     |                         |
|---------------------|---------|--------------------|----|---------------------|-------------------------|
| $L_b = 30$          | ft      | $L_p = 9.23$       | ft | $L_r = 40.01$       | ft                      |
| $\lambda = 5.86$    |         | $\lambda_p = 9.19$ |    | $\lambda_r = 22.29$ |                         |
| $\phi M_p = 366$    | ft kips |                    |    |                     | Plastic bending moment  |
| $\phi M_n = 332.86$ | ft kips |                    |    |                     | Design bending moment   |
| $M_u = 232.25$      | ft kips |                    |    |                     | Ultimate bending moment |

"The beam is adequate if  $\phi M_n \geq M_u$ "  
 "The beam is not adequate if  $\phi M_n < M_u$ "

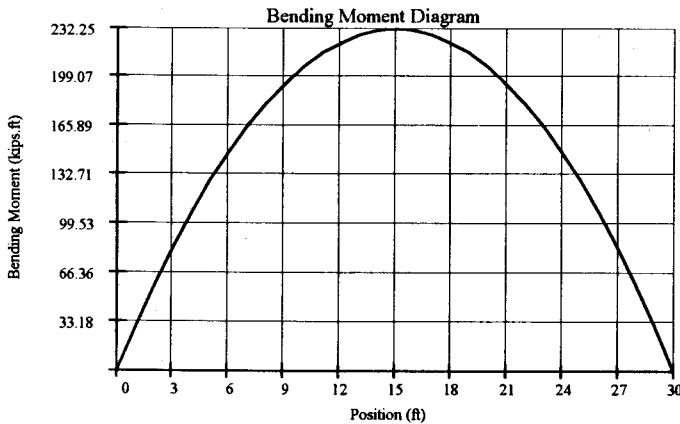


Fig. 6. Input and output results for first design trial

# Teaching Structural Steel Design Using Mathcad Program

## INPUT DATA

### Material properties data

|           |     |           |     |                             |
|-----------|-----|-----------|-----|-----------------------------|
| Fy = 50   | ksi | Fr = 10   | ksi | Yield and residual strength |
| E = 29000 | ksi | G = 11200 | ksi | Elastic and shear modulus   |

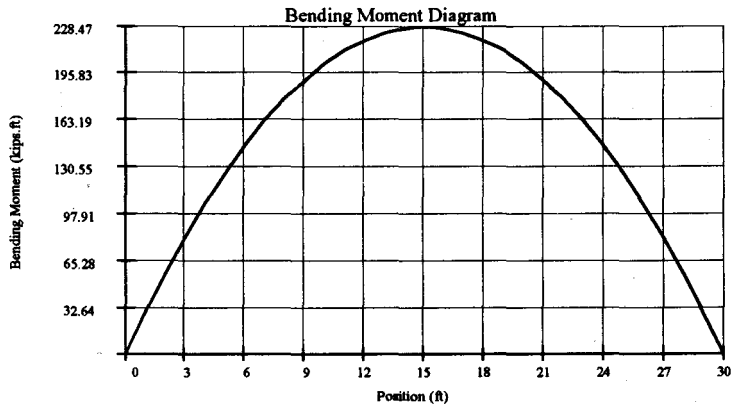
### Section properties data

|           |                 |            |                 |            |                 |  |                                   |
|-----------|-----------------|------------|-----------------|------------|-----------------|--|-----------------------------------|
| d = 9.98  | in              | A = 14.4   | in <sup>2</sup> | ow = 0.049 | kips            | ft   | Beam height, area, and own weight |
| Bf = 10.0 | in              | Tf = 0.560 | in              |            |                 |  | Flange width and thickness        |
| Ix = 272  | in <sup>4</sup> | Sx = 54.6  | in <sup>3</sup> | Zx = 60.4  | in <sup>3</sup> | Moment of inertia, section modulus, and plastic modulus (about x-axis)     |                                   |
| Iy = 93.4 | in <sup>4</sup> | J = 1.39   | in <sup>4</sup> | Cw = 2070  | in <sup>6</sup> | Moment of inertia (about y-axis), torsional constant, and warping constant |                                   |

## OUTPUT RESULTS

|               |         |                         |    |            |    |
|---------------|---------|-------------------------|----|------------|----|
| Lb = 30       | ft      | Lp = 9                  | ft | Lr = 28.37 | ft |
| λ = 8.93      |         | λp = 9.19               |    | λr = 22.29 |    |
| φ Mp = 226.5  | ft kips | Plastic bending moment  |    |            |    |
| φ Mn = 173.71 | ft kips | Design bending moment   |    |            |    |
| Mu = 228.47   | ft kips | Ultimate bending moment |    |            |    |

"The beam is adequate if  $\phi Mn \geq Mu$ "  
 "The beam is not adequate if  $\phi Mn < Mu$ "



**Fig. 7. Input and output results for second design trial**

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## INPUT DATA

### Material properties data

|                   |     |                   |     |                             |
|-------------------|-----|-------------------|-----|-----------------------------|
| $F_y \approx 50$  | ksi | $F_r \approx 10$  | ksi | Yield and residual strength |
| $E \approx 29000$ | ksi | $G \approx 11200$ | ksi | Elastic and shear modulus   |

### Section properties data

|                     |                 |                     |                 |                    |                                 |  |
|---------------------|-----------------|---------------------|-----------------|--------------------|---------------------------------|--|
| $d \approx 12.19$   | in              | $A \approx 17.0$    | in <sup>2</sup> | $ow \approx 0.058$ | $\frac{\text{kips}}{\text{ft}}$ | Beam height, area, and own weight  |
| $B_f \approx 10.01$ | in              | $T_f \approx 0.640$ | in              |                    |                                 | Flange width and thickness   |
| $I_x \approx 475$   | in <sup>4</sup> | $S_x \approx 78$    | in <sup>3</sup> | $Z_x \approx 86.4$ | in <sup>3</sup>                 | Moment of inertia, section modulus, and plastic modulus (about x-axis)     |
| $I_y \approx 107$   | in <sup>4</sup> | $J \approx 2.10$    | in <sup>4</sup> | $C_w \approx 3570$ | in <sup>6</sup>                 | Moment of inertia (about y-axis), torsional constant, and warping constant |

## OUTPUT RESULTS

|                     |         |                    |    |                     |                         |
|---------------------|---------|--------------------|----|---------------------|-------------------------|
| $L_b = 30$          | ft      | $L_p = 8.87$       | ft | $L_r = 26.96$       | ft                      |
| $\lambda = 7.82$    |         | $\lambda_p = 9.19$ |    | $\lambda_r = 22.29$ |                         |
| $\phi M_p = 324$    | ft kips |                    |    |                     | Plastic bending moment  |
| $\phi M_n = 232.17$ | ft kips |                    |    |                     | Design bending moment   |
| $M_u = 229.68$      | ft kips |                    |    |                     | Ultimate bending moment |

"The beam is adequate if

$$\phi M_n \geq M_u$$

"The beam is not adequate if

$$\phi M_n < M_u$$

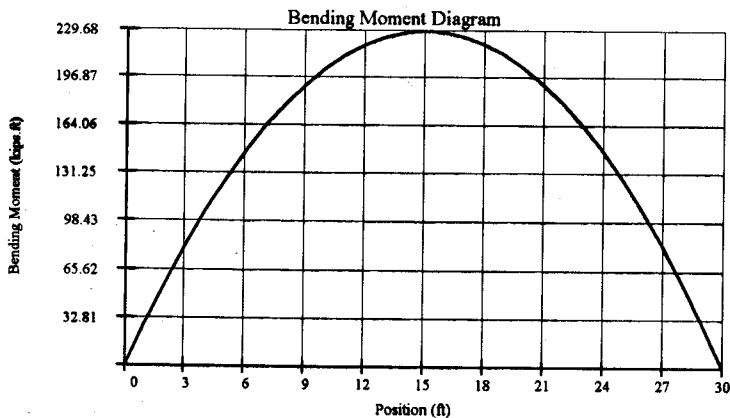


Fig. 8. Input and output results for third design trial

## Teaching Structural Steel Design Using Mathcad Program

### CONCLUSIONS

Mathcad contains tools, which enhances and supplements student's understanding of the course material. The versatility, accessibility, and ease of use make Mathcad a platform for creating learning modules for technically based courses. Mathcad contains the capabilities for classroom computation with a greater degree of accuracy, reliability, and presentation quality. In addition, its speed at repetitive tasks, and its programmability, makes new learning strategies possible. Mathcad programs take time for students to develop, but with many benefits in return. They create opportunities for meaningful understanding of technical material. A well-designed Mathcad program can engage students to explore and discover the subject, drawing them deeper into the secrets that it holds.

### REFERENCES

1. Mathcad, MathSoft Inc., 101 Main Street, Cambridge, Massachusetts 02142, USA (1995).
2. Al-Ansari M. and Senouci A., "Mathcad: Teaching and Learning Tool for Reinforced Concrete Design," International Journal of Engineering Education, Vol. 15, No.1, pp. 64-71 (1999).
3. AISC, Manual of Steel Construction-Load and Resistance Factor Design, American Institute of Steel Construction, Inc., Detroit, USA (1995).
4. McCormac J. C, Structural Steel Design-LRFD Method, HarperCollins College Publishers, Inc., 10 East 53rd Street, New York, NY 10022, USA (1995).
5. Salmon C. G. and Johnson J. E., Steel Structures - Design and Behavior, HarperCollins Publishers, Inc., 10 East 53rd Street, New York, NY 10022, USA(1992).
6. Segui, W. T., LRFD Steel Design, PWS Publishing Company, 20 Park Plaza, Boston, MA 02116-4324, USA(1994).
7. Smith, J.C., Structural Steel Design, John Wiley & Sons, Inc., New York (1996)

APPENDIX

Some Conversion factors, between customary and SI metric unit, Useful in Structural Steel Design

|                 | To Convert          | To                  | Multiply by |
|-----------------|---------------------|---------------------|-------------|
| Forces          | kip force           | kN                  | 4.448       |
|                 | lb                  | N                   | 4.448       |
|                 | kN                  | kip                 | 0.225       |
| Stresses        | ksi                 | MPa                 | 6.895       |
|                 | psi                 | MPa                 | 0.006985    |
|                 | MPa                 | ksi                 | 0.1450      |
|                 | MPa                 | psi                 | 145.0       |
| Moments         | ft.kip              | kN.m                | 1.356       |
|                 | kN.m                | ft.kip              | 0.7376      |
| Uniform Loading | kip/ft              | kN/m                | 14.590      |
|                 | kN/m                | kip/ft              | 0.06852     |
|                 | kip/ft <sup>2</sup> | kN/m <sup>2</sup>   | 47.88       |
|                 | psf                 | N/m <sup>2</sup>    | 47.88       |
|                 | kN/m <sup>2</sup>   | kip/ft <sup>2</sup> | 0.02089     |